

### DS/EN 1993-1-3 DK NA:2019

National Annex to

**Eurocode 3: Design of steel structures –** 

Part 1-3: General rules – Supplementary rules for cold-

formed members and sheeting

#### **Foreword**

This National Annex (NA) is a revision of DS/EN 1993-1-3 DK NA:2013 and replaces the latter as from 2019-09-09. For a transition period until 2019-12-31, this National Annex as well as the previous National Annex will be applicable.

Text has been added under Clause 2(3)P Basis of design in relation to level of checking.

This NA lays down the conditions for the implementation in Denmark of EN 1993-1-3 for construction works in conformity with the Danish Building Regulations.

This NA applies to construction works covered by section 16(1) of the Danish Building Regulations as well as to construction works covered by sections 24 to 27 of the Danish Building Regulations.

#### This NA includes:

- an overview of possible national choices and clauses containing complementary information;
- national choices;
- (non-contradictory), complementary information which may assist the user of the Eurocode.

For structures covered by sections 24 to 27 of the Danish Building Regulations BR18, or not covered by the Danish Building Regulations, levels of checking may still be used for the calculation of structures in ultimate limit states. For structures covered by section 16(1) of the Danish Building Regulations, levels of checking cannot be applied.



# Overview of possible national choices and clauses containing complementary information

The list below identifies the clauses where national choices are possible and the applicable/not applicable informative annexes. Furthermore, clauses giving complementary information are identified. Complementary information is given at the end of this document.

Clause	Subject	National choice 1)	Complementary information <sup>2)</sup>	
2(3)P	Basis of design	National choice	Complementary information	
2(5)	Basis of design	Unchanged		
3.1(3), Note 1	Materials, General	Unchanged		
3.1(3), Note 2	Materials, General	Unchanged		
3.2.4(1)	Materials, Structural steel	Unchanged		
5.3(4)	Structural analysis, Structural modelling for analysis	Unchanged		
8.3(5)	Connections with mechanical fasteners	National choice		
8.3(13)	Connections with mechanical fasteners	Unchanged		
8.3(13) Table 8.1	Design resistances for blind rivets	National choice		
8.3(13) Table 8.2	Design resistances for self-tapping screws	National choice		
8.3(13) Table 8.3	Design resistances for cartridge fired pins	National choice		
8.3(13) Table 8.4	Design resistances for bolts	National choice		
8.4(5)	Spot welds	National choice		
8.5.1(4)	Lap welds	National choice		
9(2)	Design by testing	Unchanged		



Clause	Subject	National choice	Complementary information 2)
10.1.1(1)	Special considerations for purlins, liner trays and sheeting – Beams restrained by sheeting	Unchanged	
10.1.4.2(1)	Special considerations for purlins, liner trays and sheeting – Beams restrained by sheeting	Unchanged	
A.1(1) Note 2	Testing procedures, General	Unchanged	
A.1(1) Note 3	Testing procedures, General		Complementary information
A.6.4(4)	Testing procedures, Assessment of test results	Unchanged	
E(1)	Simplified design for purlins	Unchanged	

<sup>1)</sup> Unchanged: No national choice is made, and recommendations in the standard are followed.

<sup>&</sup>lt;sup>2)</sup> Complementary information: Non-contradictory, complementary information on how to use the Eurocode



#### **National choices**

#### 2(3)P Basis of design

The below expressions for  $\gamma_{Mi}$  are used, including the factor ( $\gamma_0$ ) on the partial factors for strength parameters and resistances, cf. National Annex to EN 1990, Table A1.2(B+C):

$$\gamma_{M0} = 1, 1 \cdot \gamma_0 \cdot \gamma_3$$

$$\gamma_{M1} = 1, 2 \cdot \gamma_0 \cdot \gamma_3$$

$$\gamma_{M2} = 1, 35 \cdot \gamma_0 \cdot \gamma_3$$

The factor  $\gamma_0$  takes into account the combination of actions, cf. National Annex to EN 1990, Table A1.2(B+C).

Limit state	STR/GEO				STR
Combination of actions	1	2	3	4	5
<b>70</b>	1,0	1,0	$K_{ m FI}$	$K_{ m FI}$	1,2· <i>K</i> <sub>FI</sub>

The factor  $\gamma_3$  takes account of the level of checking of the product. The reduced level of checking is not used.

Extended level of checking:  $\gamma_3 = 0.95$ 

Normal level of checking:  $\gamma_3 = 1,00$ 

For structures covered by section 16(1) of the Danish Building Regulations, the extended level of checking cannot be applied, and  $\gamma_3$  is taken as 1,00.

The partial factors are determined in accordance with the National Annex to EN 1990, Annex F, where  $\gamma_M = \gamma_1 \gamma_2 \gamma_3 \gamma_4$ .

 $y_1$   $y_1$ : takes into account the type of failure;

 $y_2$   $y_2$ : takes into account the uncertainty related to the design model;

 $y_3$  takes into account the extent of checking;

y<sub>4</sub> takes into account the variation of the strength parameter or re-

sistance.

When determining  $\gamma_1$ , the following types of failure have been assumed:

M<sub>0</sub>: Warning of failure with residual resistance

 $\gamma_{\rm M1}$ : Warning of failure without residual resistance

<sub>1</sub>M<sub>2</sub>: No warning of failure

For accidental and seismic design situations the following values are used:

 $\gamma_{\rm M0} = 1.0$ 

 $\gamma_{\rm M1} = 1.0$ 

 $\gamma_{\rm M2} = 1.0$ 



#### 8.3(5) Connections with mechanical fasteners

The following value is used:

$$\gamma_{M2} = 1.35 \cdot \gamma_0 \cdot \gamma_3$$

#### 8.3(13), Table 8.1, Design resistances for blind rivets

The resistance with respect to the forms of failure indicated should be determined by testing, or documented resistances provided by the supplier should be applied.

#### 8.3(13), Table 8.2, Design resistances for self-tapping screws

The resistance with respect to the forms of failure indicated should be determined by testing, or documented resistances provided by the supplier should be applied.

#### 8.3(13), Table 8.3, Design resistances for cartridge fired pins

The resistance with respect to the forms of failure indicated should be determined by testing, or documented resistances provided by the supplier should be applied.

#### 8.3(13), Table 8.4, Design resistances for bolts

The resistance with respect to the forms of failure indicated should be determined by testing, or documented resistances provided by the supplier should be applied.

#### **8.4(5) Spot welds**

The following value is used:

$$\gamma_{M2} = 1.35 \cdot \gamma_0 \cdot \gamma_3$$

#### 8.5.1(4) Lap welds

The following value is used:

$$\gamma_{M2} = 1.35 \cdot \gamma_0 \cdot \gamma_3$$



## (Non-contradictory), complementary information

#### 2(3)P Basis of design

For traditional cold-formed members and sheeting not used for resisting geotechnical actions, combinations of actions 3, 4 and 5 can be disregarded.

#### A.1(1), Note 3, Testing procedures, General

For each case an assessment should be made of how available test results are converted into values corresponding to results of tests carried out according to Annex A.